



In USFOS





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1 Introduction

The normal foundation of offshore jacket structures consists of single or groups of steel piles driven into the soil. This document describes how to define the non-linear pile-soil foundation in USFOS.

2 Typical Model

A typical USFOS model is shown in Figure 2-1. The part of the FE model below the seabed is generated by USFOS based on the definition of the soil and piles. The soil utilization as well as the soil strength is visualized, (see Figure 2-2).



Figure 2-2 Soil disc diameter reflects the relative soil strength.



3 Pile-Soil modelling.

3.1 Basic concept

Figure 3-1 describes one pile, which penetrates 5 different (real) soil layers. The structural model (gold) ends at the mud line and is attached to the pile at the interface node (green). The pile tip node has to be defined by the user as an ordinary "NODE" (with xyz coordinates) and is used by USFOS to define the pile. The pile is defined from the pile head towards the tip, (ie: the local pile X-axis goes downwards).

Based on the definition of the pile and soil, USFOS will generate the necessary nodes and elements to describe the foundation. The nodes are inserted in the centre of each soil layer, and one soil element representing the entire soil layer is attached to this node. Between these centre nodes, ordinary USFOS beam elements are generated (automatically) with actual diameter, thickness and steel material properties.



Figure 3-1 Description of one pile penetrating 5 different soil layers.



3.2 Coordinate system

The localization of the different soil layers is specified with the top/bottom coordinates of each layer. Following convention is used with respect to coordinate systems:

- □ Node Coordinates follows the normal Global system. (Pile Tip Nodes)
- □ The Z-coordinate of the mud line is specified in the Global Coordinate system (the same system as used for the structural model).
- □ All Soil Layer Z-coordinates are given *relative* to Z_{MUD} . This means that the upper layer always starts with Z=0.0 (in Soil Coordinates).
- □ NOTE that the soil Z coordinate points upwards.



Figure 3-2 The soil layers are defined by the Z-coordinates relative to the Mud Line..

For each soil layer, following information has to be defined:

- \Box Z_{TOP} and Z_{BOTTOM}, (relative to mud line)
- □ Soil properties (P-Y, T-Z and Q-Z)

The soil properties could be defined as user-defined curves defining strength per unit length of the pile, (one curve per property), or by soil data, (sand and clay parameters).



3.3 Defining Mesh density

The definition of the soil layers defines implicitly the finite element mesh density of the piles. The nodes are inserted in the centre of each layer, and the soil element is attached to this node, (one node "soil-to-ground" element). Between these centre nodes, ordinary beam elements are inserted. The pile diameter/thickness is either constant from pile head to tip (default) or varying (additional information is required).

If soil layers are relatively thick, the length of the pile element could become too long, (and, for example, may buckle in compression). In order to get appropriate length of the pile elements, it's just to defined "soil layers" with wanted thickness, but referring to the same soil properties several times.

Figure 3-3 describes a case with 3 different real soil layers, and where the model consists of 7 layers in order to reduce the pile element length. Layer 1-3 refers to the same Soil-1 data, layers 4-6 refers to Soil-2 data, while layer 7 refers to Soil-3 data.

If the *user defined soil curves* are used, identical strength per unit length of the pile will be used for the "layers" within each "Soil".

If the soil is defined by it's geotechnical properties (sand/clay etc), USFOS will account for the increasing overburden pressure within each soil. The layers within the same soil type will then get increasing strength for increasing depths, (overburden pressure at centre node is used).



Figure 3-3 Sub dividing the mesh within thick soil layers.



4 Examples.

4.1 User Defined Soil Curves

This example describes the use of "User Defined" curves. User defined curves means that the soil data are describes in terms of Pile Strength per unit length of the pile for each soil layer.

The simple pile example is shown in Figure 4-1. The pile is exposed to a vertical (compression) force, which introduces buckling, (because the end bearing capacity is high and the lateral resistance is low in this *designed* case).

The pile plastic utilization as well as the soil utilization is visualized on the deformed pile. In order to easier see the failure modes, the utilization fringes are used as follows:

- □ Soil utilization dominated by T-Z : Symmetric colours
- □ Soil utilization dominated by P-Y : Unsymmetrical, Exposed side becomes red.

In addition, the deformation are used as follows:

The soil disc inner circle follows the vertical movements of the pile, but is fixed in the horizontal direction. In connection with lateral failure, a "gap" opens on the unexposed side, to easier see the failure mode.



Figure 4-1 Buckling of Single pile. Visualization of utilization and deformations.

The soil strength is defined by the general non-linear curve commands ElPlCurve (Elasto-Plastic Curve) and HypElastic (Hyper Elastic). These curves are defined by discrete points, Force and Displacement, starting in 3rd quadrant.

- \Box Force : Pile resistance per unit length of the pile [N/m]
- Displacement : Corresponding deformation

The forces are referred to a *certain pile diameter*, and this diameter is specified in the SOILCHAR record (D_{REF}). If another pile diameter is used, USFOS will scale the capacity accordingly (linear scaling).



Figure 4-2 Definition of Curves. Specify points from Quadrant 3 towards Q1.

Figure 4-3 describes typical pile-soil capacity curves. The P-Y and T-Z curves are typically symmetric (equal capacity in both directions), while the Q-Z typically has no resistance for pile tension. Curve points in Quadrant-3 represent the tension and Q1 represents compression, (relevant for QZ and TZ). The P-Y data are uni-directional, but are always given as a symmetric curve. Table 4-1, describes the complete pile-soil input for this simple example.



Figure 4-3 Typical T-Z, P-Y and Q-Z curves.

[m]





Table 4-1 USFOS commands defining a single pile + user-defined soil.

4.2 How to define soil capacity curves

This example describes how to create the pile-soil curves based on available Geotechnical data. Each curve must be given a unique Material ID no, and it is recommended to use a simple system, (to avoid chaos). In the example below, the following system is used:

P-Y curves	:	1,000 series,	1001, 1002, 1003,etc
T-Z curves	:	2,000 series,	2001, 2002, 2003,etc
O-Z curves	:	3,000 series,	3001, 3002, 3003,etc

1001 is P-Y data for soil layer no 1, 1002 for layer no 2 etc.

4.2.1 P-Y curve, (Lateral Pile Resistance)

Table 4-2 describes typical lateral pile resistance data (P-Y) as they are received from geotechnical consultants. The lateral resistance, P is given as kN per unit length of a pile with diameter 2.438m. The corresponding lateral displacements, Y, are given in millimetre. Seven points are defined.

The P-Y data are given for some depths in this example, an one arbitrarily chosen layer is described in detail.

USFOS is using force per unit length of pile as input, and in the example, data should be given to USFOS in pure SI units (N, m). The scaling of the data then becomes:

Forces (P) : Multiply the tabulated values with 1000 to obtain Newton
Displacement (Y) : Divide the tabulated values with 1000 to obtain meter.

Depth	<i>P1</i>	P2	<i>P3</i>	<i>P4</i>	P5	<i>P6</i>	P 7
(<i>m</i>)	Y1	Y2	¥3	¥4	¥5	¥6	¥7
0.00	0.00	3.36	4.83	7.31	10.53	14.63	14.63
	0	12.19	36.57	121.9	365.7	975.2	2438
7.50	0.00	53.86	77.28	117.09	168.61	234.18	234.18
	0	12.19	36.57	121.9	365.7	975.2	2438
22.50	0.00	239.72	343.94	521.12	750.42	1042.25	1042.25
	0	6.095	18.285	60.95	182.85	487.6	2438
27.00	0.00	277.57	398.25	603.41	868.90	1206.81	1206.81
	0	6.095	18.285	60.95	182.85	487.6	2438
34.50	0.00	371.82	533.48	808.30	1163.95	1616.60	1616.60
	0	4.2665	12.7995	42.665	127.995	341.32	2438

Table 4-2 P-Y Data. Lateral Strength, P [kN/m] and Displacement, Y [mm] for a D=2.438m pile.

USFOS assumes that the specified curve data is valid for a soil *layer*, specified with the top/bottom Z-coordinate (see page 4), It is therefore important to check how the geotechnical data should be used.

In the present example, it is assumed that the strength for $Depth_I$ is valid to $Depth_{I+I}$. For example that P values in the row for depth 22.50 is valid for the layer between depth 22.50 and 27.00.

1.1	Matin P [N/m]	Y[m]
ElPlCurve	1003 -1042*1000	-2438/1000
	-1042*1000	-488/1000
	-750*1000	-183/1000
	-521*1000	-61/1000
	-344*1000	-19/1000
	-240*1000	-6/1000
	0.0	0.0
	240*1000	6/1000
	344*1000	19/1000
	521*1000	61/1000
	750*1000	183/1000
	1042*1000	488/1000
	1042*1000	2438/1000
'		

Table 4-3 Curve definition for Shear Strength for layer with Z=-15.00 to Z=-19.22.

SOILCHAR	Soil_ID Type 1 <i>Curves</i>	Z_mud 0.000	D_ref 2.438	F_Fac 1.000	L_Fac 1.000	
1		Z_top 0.00 -7.50 -22.50 -27.00	Z_bott -7.50 -22.50 -27.00 -34.50	P-Y 1001 1002 1003 1004	T-Z 0 2002 2003 2004	Q-Z 0 0 0 0

Table 4-4 Highlight reference to P-Y curve 1003.

In the example, the 1,000 series is used for P-Y data, and since the selected layer is the 3^{rd} , the material ID becomes 1003

The P-Y data are symmetric, and the Elasto-Plastic Curve data (ElPlCurve) is therefore given with same data in 1^{st} and 3^{rd} quadrant, (see also Figure 4-3).

It should be noted that USFOS input reader may executed simple mathematical expressions, which, in this case means increased readability vs. the original soil data.

The Material 1003 should be referred to in the SoilChar as P-Y curve, see Table 4-4.



4.2.2 T-Z curve, (Shear Capacity)

Shear strength data (T-Z) could be available in different formats: Resultant strength per unit length of a given pile, or as shear strength per unit surface area of a pile.

The first alternative could be used directly (if necessary, convert units to N and m), while the 2^{nd} alternative needs some computation.

Below, some typical shear strength data is specified in the tables. The shear strength is given for same displacement levels for all depths, (see Table 4-5). Table 4-6 describes the corresponding shear strength for the different depths. Data are derived from a pile with diameter 2.438m

The displacement is given in millimetre, while the shear strength is given in kPa.

Z1	Z2	Z3	Z4	Z5	Z6	Z 7	Z8
0.0	3.9	7.6	13.9	19.5	24.4	48.8	73.1

Table 4-5 Displacement, Z, [mm].

Depth	T1	T2	T3	T4	T5	T6	T7	T8
0.00	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1.22	0.0	0.5	0.9	1.3	1.5	1.7	1.2	1.2
13.78	0.0	3.5	5.9	8.8	10.5	11.7	8.2	8.2
15.00	0.0	3.8	6.4	9.5	11.4	12.7	8.9	8.9
19.22	0.0	6.9	11.5	17.2	20.6	22.9	16.0	16.0
25.78	0.0	9.0	15.0	22.4	26.9	29.9	20.9	20.9

Table 4-6 Shear Strength, T, [kPa].

Since USFOS requires force per length unit of a pile, the T data have to be converted as follows:

 $T_{PerLength} = T \pi D$

Where D is the outer diameter of the actual pile. In the actual case, the reference diameter is 2.438m and T is given in kPa. Since the strength curves should be given in force [N] per unit length of the pile [m], the computation is as follows:

 $T_{PerLength} = T \pi 2.428 x 1000 [N/m].$

Further, USFOS assumes that the specified curve data is valid for a soil *layer*, specified with the top/bottom Z-coordinate (see above). It is therefore important to check how the geotechnical data should be used.

In the example, it is assumed that the strength for $Depth_I$ is valid to $Depth_{I+I}$. For example that T values in the row for depth 15.00 is valid for the layer between depth 15.00 and 19.22, (the layer is arbitrarily chosen).

,	MatID	T [N/m]	z[m]
ElPlCurve	2004	-8.9*1000*PI*2.438	-73.1/1000
		-8.9*1000*PI*2.438	-48.8/1000
		-12.7*1000*PI*2.438	-24.4/1000
		-11.4*1000*PI*2.438	-19.5/1000
		-9.5*1000*PI*2.438	-13.9/1000
		-6.4*1000*PI*2.438	-7.6/1000
		-3.8*1000*PI*2.438	-3.9/1000
		0.0	0.0
		3.8*1000*PI*2.438	3.9/1000
		6.4*1000*PI*2.438	7.6/1000
		9.5*1000*PI*2.438	13.9/1000
		11.4*1000*PI*2.438	19.5/1000
		12.7*1000*PI*2.438	24.4/1000
		8.9*1000*PI*2.438	48.8/1000
		8.9*1000*PI*2.438	73.1/1000
1			

Table 4-7 Curve definition for Lateral Strength for layer with Z=-22.50 to Z=-27.00.

In the example, the 2,000 series is used for T-Z data, and since the selected layer is the 4th, the material ID becomes 2004

It is assumed that the T-Z data are equal for up/down movements of the pile (tension/compression), and the curve is therefore symmetric.

It should be noted that USFOS input reader may executed simple mathematical expressions, which, in this case means increased readability vs. the original soil data.

The Material 2004 should be referred to in the Soilchar as T-Z curve, see Table 4-8.

SOILCHAR	Soil_ID Type 1 <i>Curves</i>	Z_mud 0.000	D_ref 2.438	F_Fac 1.000	L_Fac 1.000	
,		Z_top	Z_bott	P-Y	T-Z	Q-Z
		0.00	-1.22	1001	0	0
		-1.22	-13.78	1002	2002	0
		-13.78	-15.00	1003	2003	0
		-15.00	-19.22	1004	2004	0
•••••						

Table 4-8 Highlight reference to T-Z curve 2004..

4.2.3 Q-Z curve, (End Bearing)

Table 4-9 and Table 4-10 describe typical end bearing data for a pile with diameter 2.438m, (96"). The first table gives the displacements for the 7 points (Z1-Z7) while Table 4-10 gives the corresponding end bearing capacity for different depths, (from depth=68 –90.50).

The displacements are given in millimetre, while the end bearing is given in kN. It is recommended to use pure SI units, (m, N, kg), and the generated curve will be given in SI-units.

Since Q-Z data only are relevant for the soil layers where the pile tip is located, the first Q-Z curve is defined for depth = 68m (or Z = -68 in the USFOS soil coordinate system). The *suggested* Material ID to use for this soil layer is *3052*, (Q-Z, 52^{th} layer).

Z1	Z2	Z3	Z4	Z5	Z6	Z7
0.0	5	32	102	178	244	731

Table 4-9 Displacements [mm].

Depth	Q1	Q2	Q3	Q4	Q5	Q6	Q7
68.00	0.0	1260.4	2520.9	3529.2	4537.6	5041.7	5041.7
69.22	0.0	1286.0	2572.1	3600.9	4629.8	5144.2	5144.2
76.78	0.0	1444.9	2889.8	4045.7	5201.6	5779.6	5779.6
78.00	0.0	1470.5	2941.0	4117.4	5293.8	5882.0	5882.0
79.22	0.0	1496.1	2992.2	4189.1	5386.0	5984.5	5984.5
89.28	0.0	1707.5	3415.0	4781.0	6147.0	6830.0	6830.0
90.50	0.0	1733.1	3466.2	4852.7	6239.2	6932.4	6932.4

Table 4-10 End Bearing [kN].

The Q-Z data are valid for pile compression only, and the curve therefore becomes *un*-symmetric. The curves are always defined from the very left side, (Q3, ref Figure 4-2 and Figure 4-3 for general description).

The Q-Z data are defined in Newton and meter. Curve 3052 is referred to from SOILCHAR as shown below.



Table 4-11 Q-Z curve definition for soil layer no 52. Data in N and m.

		\mathbf{i}			
1					
' Soil_ID Type	Z_mud	D_ref 🔪	F_Fac	L_Fac	
SOILCHAR 1 Curves	0.000	2.438	1.000	1.000	
'	Z_top	Z_bott	P-Y	T-Z	Q-Z
	-0.05	-0.05	1002	0	0
	-0.55	-1.22	1002		0
	-1.22	-1.50	1004	2004	0
	-1.50	-2.50	1005	2005	0
	-2.50	-3.50	1006	2006	0
				\backslash	
	66 79	69 00	1050	2050	
	-68.00	-69.22	1052	2050	(3052)
	-69.22	-70.50	1053	2053	3953
-	151.22	-158.78	1056	2080	3080
-	158.78	-161.00	1056	2081	3081
,					

Table 4-12 Creating User Defined Pile-Soil Curves. Definition of Q-Z curve with ID 3052.

The same system is repeated for all relevant Q-Z layers. For layers without pile tip, just insert a zero (0).